



Floor Joist - M2

TIMBER FLOOR BEAM V5.10

Engineering Building & Infrastructure Pty Ltd

Beam:	(Floor Beam M2- Mezzanine) 180mm x 50mm Smart LVL19** (Douglas Fir) (WA only) (Single span)	
Bending:	$M(dI)^* = 0.89\text{kNm (+ve)} < \phi M(dI) = 9.87\text{kNm}$, $M^* = 0.89\text{kNm (+ve)} < \phi M = 13.85\text{kNm}$	OK (0.09,0.06)
Shear:	$V(dI)^* = 0.71\text{kN} < \phi V(dI) = 17.54\text{kN}$, $V^* = 0.71\text{kN} < \phi V = 24.62\text{kN}$	OK (0.04,0.03)
Deflection:	$\delta DL = L/-12607$ (0mm), $\delta \Psi s.LL = L/-420$ (-12mm), $\delta Total = L/-407$ (-12mm), 1kN mid. $\delta = 5.6\text{mm}$	OK (0.04,0.03)
Reactions:	(Each end) $RdI = 0.53\text{kN}$, $RlI = -1.69\text{kN}$, $R^* = 0.71\text{kN}$	

Geometry (House member affecting $\leq 25\text{m}^2$ or secondary building member)

Category =	1 (1) House, (2) Primary building elements, (3) Important	
House =	Y (Y)es,(N)o	Edge restrained = B (T)ension, (C)ompression, (B)oth
Span (L) =	5000 mm	Lay.t (Top) = 600 mm
Centres (cts) =	450 mm	Lay.b (Bottom) = 600 mm
Span type =	S (S)ingle,(D)ouble	

Loadings

Floor area (A) =	2.25 m ²	Live load type =	N (N)ormal, (S)torage, (M)anual
			AS/NZS 1170.0 - Table 4.1
Uniform dead loads			
Floor dead load (wdl) =	0.35 kPa *	450 mm +	kN/m = 0.16 kN/m
Wall dead load (wdl) =	kPa *	450 mm +	kN/m = 0.00 kN/m
Other dead load (wdl) =	kPa *	mm +	kN/m = 0.00 kN/m
Include S.Wt =	Y (Y)es,(N)o	S.Wt =	0.05 kN/m
		$\Sigma wdl =$	0.21 kN/m
Uniform live loads			
Floor live load (wll) =	-1.50 kPa *	450 mm +	kN/m = -0.68 kN/m
Partitions (wll) =	kPa *	450 mm +	kN/m = 0.00 kN/m
Alternate point live load =	-7.50 kN	Distr. to 1 members	$\Sigma wll = -0.68 \text{ kN/m}$
Point loads			
Dead load (pdl) =	kN	Position =	2500 mm from LHS
Live load (pll) =	kN	Shear using PL at support =	N (Y)es,(N)o
Short term LL factor (Ψ_{su}) =	1.00	(Ψ_{sp}) =	1.00
Long term LL factor (Ψ_{lu}) =	0.33 / 0.40 (wdl*)	(Ψ_{lp}) =	0.33 / 0.40 (pdl*)
$wdl^* = 1.35 * wdl =$	0.29 kN/m	$M(dI + \Psi_{lI})^* =$	0.89 kNm (Max at 2500mm)
$w^* = 1.35 * wdl =$	0.29 kN/m	$M^* =$	0.89 kNm (Max at 2500mm)
$pdl^* = 1.2 * pdl + 1.5 * \Psi_{lp} * pll =$	0.00 kN	$V(dI + \Psi_{lI})^* =$	0.71 kN
$p^* = 1.2 * pdl + 1.5 * pll =$	0.00 kN	$V^* =$	0.71 kN

Bending and Shear capacity - Cl 3.2.1 & Cl 3.2.5

Member =	180mm x 50mm Smart LVL19	Area (A) =	9000 mm ²
Description =	LVL19 seasoned softwood	Section modulus (Z_x) =	270 x10 ³ mm ³
Design depth (dD) =	180 mm	Stiffness (I_x) =	24.3 x10 ⁶ mm ⁴
Design width (dW) =	50 mm	Modulus of elasticity (E) =	19000 MPa - Cl 8.3
$S1 = (dD/dW)^{1.35} * (Lay.b/dD)^{0.25} =$	7.62	For bottom tension edge restrained - Cl 3.2.3.2(a)	
$k12d =$	1.000 for $pbd * S1 \leq 10$ - Cl 3.2.4	$f'b =$	75.0 MPa
$k12 =$	1.000 for $pb * S1 \leq 10$ - Cl 3.2.4	$f's =$	6.0 MPa
Strength reduction factor (ϕ) =	0.95 Table 2.1	Material constant (pbd) =	1.12 ($rbd=0.25$)
$\phi M(dI) = \phi * (k1=0.57) * k4 * k6 * k9 * k12d * f'b * Z_x =$	9.87 kNm	Material constant (pb) =	1.12 ($rb=0.25$)
$\phi M = \phi * k1 * k4 * k6 * k9 * k12 * f'b * Z_x =$	13.85 kNm	Duration factor ($k1$) =	0.80
$\phi V(dI) = \phi * (k1=0.57) * k4 * k6 * f's * (2/3 * A) =$	17.54 kN	Moisture factor ($k4$) =	1.00
$\phi V = \phi * k1 * k4 * k6 * f's * (2/3 * A) =$	24.62 kN	Temp. factor ($k6$) =	0.90
		Sharing factor ($k9$) =	1.00
		Size modifier (mod.b) =	1.00
		Size modifier (mod.s) =	1.00

Deflections

Duration factor ($j2$) =	2.0		
Ireq'd $j2.(DL + \Psi_{lI}.LL)$ (16mm) =	-0.6 x10 ⁶ mm ⁴	< Critical	$j2.(DL + \Psi_{lI}.LL) =$ -0.4 mm
Ireq'd $\Psi_{s.LL}$ (12mm) =	-24.1 x10 ⁶ mm ⁴		$\Psi_{s.LL} =$ -11.9 mm
Ireq'd Total (L/250) =	-14.9 x10 ⁶ mm ⁴		$\delta Total =$ -12.3 mm
		1kN midspan $\delta =$	5.6 mm
			Span / -12607
			Span / -420
			Span / -407