



TWO-WAY SLABS V5.02

Engineering Building & Infrastructure Pty Ltd

Geometry: (Slab SL01) 5000mm long, 5000mm wide, 250mm thick, $f'c=32\text{MPa}$
Thickness: Thickness min. ($L/250$) = 136mm, Thickness min. (inc) ($L/500$) = 149mm
Strength: Refer slab and beam design

OK

Geometry - 1 short edge discont. (Corners prevented from uplift)

Concrete strength ($f'c$) =	32 MPa	Long edge length (L_y) =	5000 mm
Slab thickness (th) =	250 mm	Short edge length (L_x) =	5000 mm
Reinf't ductility class =	L (Normal), (Low)	Continuous long edges =	2 0, 1, 2
		Continuous short edges =	1 0, 1, 2

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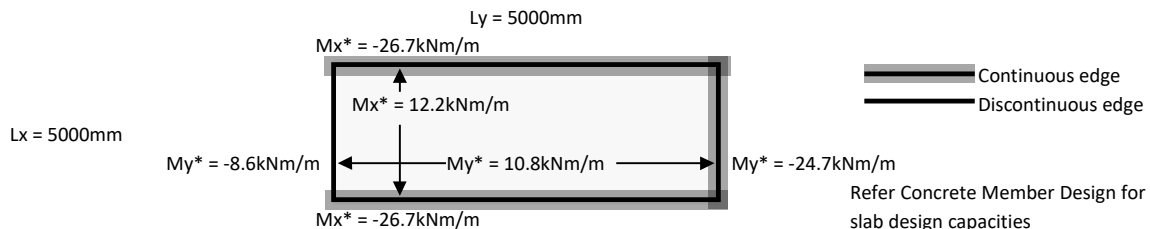
Include S.Wt =	Y (Yes), (No)	Live load type =	N (Normal), (S)orage, (R)ooft
Superimposed dead load (w_{sdl}) =	5.00 kPa	Short term LL factor (ψ_s) =	0.7 (Normal load use)
Live load (w_{ll}) =	3.00 kPa	Long term LL factor (ψ_l) =	0.4 AS/NZS 1170.0 - Table 4.1
S.Wt =	6.25 kPa		
$w_{dl} = S.Wt + w_{sdl} =$	11.25 kPa		
$F_d = 1.2*w_{dl} + 1.5*w_{ll} =$	18.00 kPa		

Analysis - Cl 6.10.3 (Class L reinforcement and/or no redistribution allowed - Table 6.10.3.2(B))

From Table 6.10.3.2(B)

$\beta_y =$	0.024	$\alpha_y =$	2.29	$L_y/L_x =$	1.00
$\beta_x =$	0.027	$\alpha_x =$	2.20		

		At discontinuous edge ($0.8*M^*$)	Cl 6.10.3.2(c)	At continuous edge ($\alpha*M^*$)	Cl 6.10.3.2(b)
$M_y^* = \beta_y * F_d * L_x^2 =$	10.8 kNm/m	8.6	kNm/m	24.7	kNm/m
$M_x^* = \beta_x * F_d * L_x^2 =$	12.2 kNm/m	9.7	kNm/m	26.7	kNm/m



Minimum strength requirements - Cl 9.1.1

$A_{st,min} = 0.19 * (D/d)^2 * f'_{ct} / f_{sy} * b * d =$	374 mm ² /m	Cl 9.1.1(b) Where supported by beams or walls on four sides
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Deflection (Deemed to conform) - Cl 9.4.4.2

Warning - Compression steel in tension, Asc ignored for kcs

SL92 (main wires in main direction)

Tensile steel (A_{st}) =	290 mm ² /m		
Yield strength of steel (f_{sy}) =	500 MPa	$p = A_{st}/d_s =$	0.00135
Cover to bottom steel (cover) =	30 mm	$p_c = A_{sc}/d_s =$	0.00134
Nominal bar size (dia) =	8.6 mm	$d_c/d_s =$	0.13908
Depth to tensile steel ($d_s = th - cover - dia/2$) =	216 mm	$n = E_s/E_c =$	6.99
Depth to comp. steel (d_c) =	30 mm from top	Neutral axis (k_u) =	0.129
Compression steel (A_{sc}) =	290 mm ² /m in positive region	Cracked NA = $k_u * d_s =$	27.8 mm
$k_{cs} = [2 - 1.2 * (A_{sc}/A_{st})] \geq 0.8 =$	2.00	Cl 8.5.3.2	
Effective design service load ($F_{d,ef}$) =	38.25 kPa	Cl 9.4.4.1(a)	Use $f_{cmi} =$ N (Yes), (No)
Incremental ($F_{d,ef,inc}$) =	27.00 kPa	Cl 9.4.4.1(b)	$f_{cmi} = f'c =$ 32.0 MPa
Mod. of elasticity ($E_c = \rho^{1.5} * 0.043 * f_{cmi}$) =	28600 MPa $\pm$ 20%	Cl 3.1.2	Concrete density (ρ) = 2400 kg/m ³ Cl 3.1.3
$k_3 =$	1.00	Cl 9.4.4.2	
$k_4 =$	3.40	Table 9.4.4.2	
$L_{ef}/\Delta =$	250	Table 2.3.2	
$L_{ef}/\Delta_{inc} =$	500	Table 2.3.2	
$d_{min} =$	102 mm	OK	Thickness min. = 136 mm
$d_{min,inc} =$	115 mm	OK	Thickness min. (inc) = 149 mm